

# **Technical Note**

Title: Cantilever deflections

**Date:** 06/12/2018

Versions: All

Program: MasterFrame, PowerPad, Beam Designers

#### Introduction

Pure "encastre" cantilevers rarely exist outside the classroom. Modelling your beam as having a fixed support, can grossly underestimate deflections.

### Example

A typical example is a cantilever beam with a back beam to provide fixity and supported on 2 knife-edge walls.

-201 @ 0		
533x210 VB 101		
-201 @ 0		
	 -38.25	
533x210 UB 101	533x210 UB 101	

LOADING at ULS (All Spans Loaded)



BM at ULS and Deflection at SLS (all spans Loaded)

Notice, that while the cantilever moments are the same, the end of cantilever Deflection is over twice as large (1:2.44 ratio) due to the rotation of the support.

Now if we then factor in the use of patterned loading with max on the cantilever & minimum on the back span, things get worse.



Deflection at SLS (Max, Min)

The cantilever deflection increases to 41.6 mm, a 1: 2.97 ratio.

# Conclusion.

Always model all the supporting members of a cantilever.

### Example models



## **Steel Design**

The MasterSeries tries to identify cantilevers but, in this case, the high free end shear would prevent it from deciding it is a cantilever.

You Must always check if it is set as a cantilever. Modify the Deflection limit as required, and choose an appropriate unrestrained effective length.

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C1	0.000	Kt-factor	0.000	Spacing	0	F-Holes No.	0.00	(1.5 L	~
C2	0.000	Lx	0.000	LV	0.000	F-Diameter	0	As End1	~
C3	0.000	Ly	0.000	FL/UL	1.200	W-Holes No.	0	AutoSelect	~
zg	0.00	Kx-factor	1.000	Def Limit	180	W-Diameter	0		
Cantilever Warping Free  V		Ky-factor	1.000	Additional load	ls	Bolts in Row No.	0	Top Flange in Tension	~
		Theta Limit	180.00	Print both Simplified & More Exact		Bolt Pitch P1	0		
Cantilever Warping Free		Lamda Limit	180.00	AutoChange Beam to C&M		Bolt edge dist.	0	Web in Tension	~